

PRECAST BOX CULVERTS ON ZION HILL ROAD AT LOW WATER CROSSING

Guadalupe County Auditor's Office 307 W. Court, Suite 205 Seguin, Texas 78155 830-303-4188 Ext. 1328

Bid Title: PRECAST BOX CULVERTS ON ZION HILL ROAD AT LOW WATER CROSSING

Bid No.: 16-4990A

Addendum No.: 1

Issued: July 29, 2016

TO BIDDER: This addendum is an integral part of the bid package under consideration by you as a Bidder in connection with the subject matter herein below identified. Guadalupe County deems all sealed bids to have been proffered in recognition and consideration of the entire bid package – <u>including all issued addenda</u>.

Receipt of this present Addendum by a Bidder should be evidenced by returning it signed as part of the Bidder's sealed bid proposal. If the Bid has already been received by the County Judge's office, Bidder should return this Addendum in a separate sealed envelope, which is clearly marked with the Bid Title, Bid Number and Opening Date and Time, stated as follows:

ADDENDUM TO: Precast Box Culverts on Zion Hill Road at Low Water Crossing

Bid No. 16-4990A

2:00 p.m., August 9, 2016

REASON FOR ISSUANCE OF THIS ADDENDUM:

- Correction to specifications, page 7, Design Engineer and page 15, 1) General Considerations, a) Definitions, iii)
 - a) M&S Engineering not Wilson Engineering
- 2. Clarification to Specifications
 - a) No Bid Bond required, only performance and payment bonds (see page 6, Performance and Payment Bonds)
 - b) No warranty required (see page 20, Procedures at Final Acceptance, Re-inspection Procedure, "Upon receipt of Contractor's notice that work has been completed, including punch-list items resulting from earlier inspections and excepting incomplete items delayed because of acceptable circumstances, Owner will re-inspect work. Upon completion of re-inspection, Owner will either recommend final acceptance and final payment, or advise Contractor of work not completed or obligations not fulfilled as required for final acceptance. If necessary, procedure will be repeated.")
 - c) See attached documents for clearer images, as shown in bid specifications, from page 42 through page 48.
 - d) No liquidated damages will be assessed <u>and</u> project completion no later than November 30, 2016 (see page 35, Bid Submission Form, 4.0)

The information included herein is hereby incorporated into the documents of this present bid matter and supersedes any conflicting documents or portion thereof previously issued.

Receipt of this Addendum is hereby acknowled	ged by the undersigned Bidder.
Authorized Signature (Bidder)	
Title of Person Signing Above	
Typed Name of Firm, Corporation	



ENGINEERING

M&S CIVIL I ELECTR TEXAS RECISTRED ENTERED

ZION HILL LOW WATER CROSSING EXHIBIT **GUADALUPE COUNTY** CONDITIONS PROPOSED

OB: GUAD OTHER REVISIONS:
DELTA DESCRIPTION

EXH 001

INSTALL 2~9x5 BOX CULVERT CENTERED ON FLOWLINE (PRE-CAST TXDOT DETAIL SCP-9) REMOVE AND REPLACE HATCHED AREA, SEE NOTE 2 5.00 0+00 **ZION HILLS** SAWCUT FOR SMOOTH TIE IN SAWCUT FOR SMOOTH TIE IN 5.00' - INSTALL METAL BEAM GUARD FENCE AND TERMINALS AT ALL END (PER TXDOT DETAILS GF(31) LS-14 AND SGT (8)31-14 -INSTALL FLARED WINGS FOR SKEWED BOX CULVERTS (TXDOT DETAL FW-S) 120.00'

ALL EXISTING CULVERT CROSSINGS SHALL BE REMOVED AT CROSSING AND DISPOSED OF PROPERLY. ALL ROADWAY BETWEEN 0+00 AND

1+37 TO BE REMOVED.

2. ROADWAY ELEVATION SHOULD BE BUILT UP OVER CULVERT CROSSING FROM STA 0+00 TO STA 1+37. CONSTRUCT NEW ROADWAY AT CENTERLINE ELEVATION 96.50 FOR ENTIRE DISTANCE (CROWN ROAD 2% FROM CENTERLINE TO OUTSIDE EDGES). SUBGRADE SHALL BE MOISTURE CONDITIONED TO 95% DENSITY. INSTALL 10" FLEXIBLE BASE (TXDOT TYPE 1 OR 2) AND 2" HMAC, TYPE D AS SURFACE COURSE (OR MATCH LIKE KIND AND THICKNESS OF EXISTING ROADWAY).

3. ELEVATIONS AND STATIONING REFERENCED FROM DATA PROVIDED BY GUADAL UPE COLINTY. THIS INFORMATION REFERENCE A RENCEMARK

GUADALUPE COUNTY. THIS INFORMATION REFERENCE A BENCHMARK ELEVATION = 100.00 AND IS NOT AN ACTUAL ELEVATION

- UTILITY LOCATIONS ARE UNKNOWN IN THIS AREA. PLEASE CALL 1-800-DIG-TESS PRIOR TO EXCAVATING.
 GRADE NO MORE THAN 3:1 SIDESLOPES WITHIN ROW TO TIE BACK TO
- EXISTING GRADE IN PARKWAY.

Zion Hill Rd: 1/08/2016

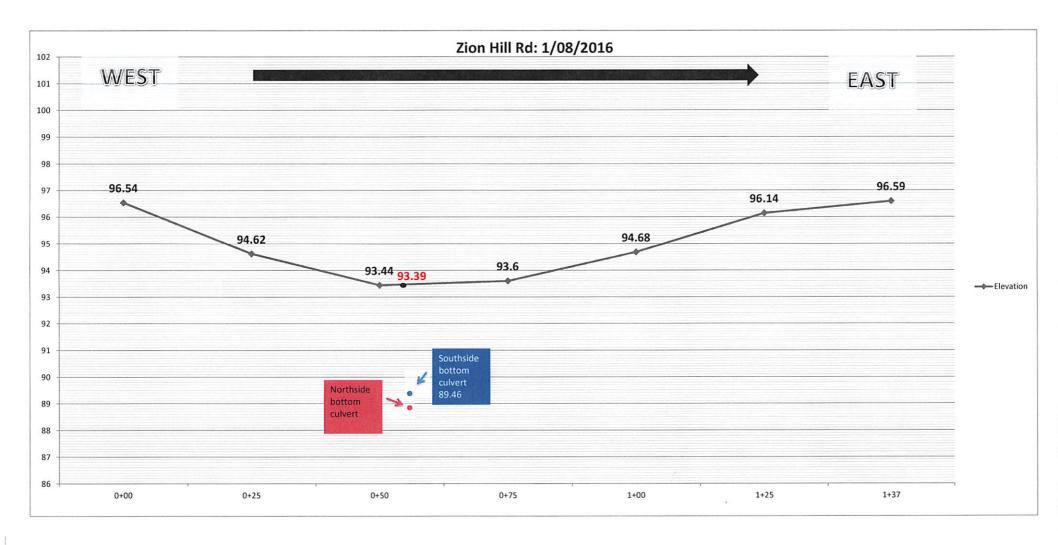
West ---> East

small bridge just past Boyd Rd before you reach 2324 address

Benchmark:South East Side Utility Pole marked with nail and green tape

STARTING BENCHMAI	RK: 100	HI: 101.29					
Station:	<u>Elevation</u>	Center of Bridge					
0+00	96.54	North Side	South Side				
0+25	94.62	Edge of Road: 8.36	Edge of Road: 8.02				
0+50	93.44	92.93	93.27				
0+55	93.39	Bottom of Culvert: 12.31	Bottom of Culvert: 11.83				
0+75	93.6	88.98	89.46				
1+00	94.68						
1+25	96.14		(6)				
1+37	96.59						

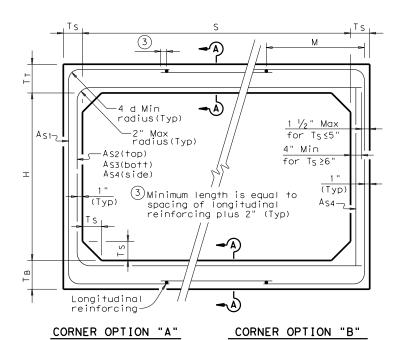




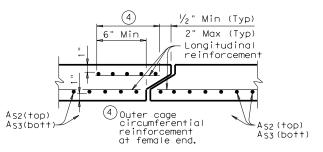
SEC	TION	DIME	NSIO	NS	E				REII	NFORC	ING (i	n²/f†)	2		1
S	Н	T _T	T _B	T _S	Fill Height	(Min)	A _{S1}	A _{S2}	A _{S3}	A _{S4}	A _{S5}	A _{S6}	A _{S7}	A _{S8}	Lift Weigh (Tons
(f+)	(f+)	(in)	(in)	(in)	(f+)	(in)									
9	5	9	9	9	<2	-	0.28	0.38	0.31	0.22	0.22	0.22	0.22	0.22	14.6
9	5	9	9	9	2<3	54	0.32	0.38	0.34	0.22	-	-	-	-	14.6
9	5 5	9	9	9	3-5 10	49 49	0.25	0.30	0.30	0.22	-	-	-	-	14.6
9	5	9	9	9	15	44	0.36	0.43	0.45	0.22	_	_	_	_	14.6
9	5	9	9	9	20	44	0.47	0.56	0.57	0.22	-	-	-	-	14.6
9	5	9	9	9	25	44	0.58	0.69	0.71	0.22	-	-	-	-	14.6
9	6	9	9	9	<2	-	0.25	0.40	0.34	0.22	0.22	0.22	0.22	0.22	15.5
9	6	9	9	9	2<3	54	0.29	0.41	0.38	0.22	-	-	-	-	15.5
9	6	9	9	9	3-5	49	0.23	0.33	0.33	0.22	-	-	-	-	15.5
9	6	9	9	9	10	49	0.26	0.35	0.37	0.22	-	-	-	-	15.5
9	6	9	9	9	15	44	0.33	0.46	0.48	0.22	-	-	_	-	15.5
9	6	9	9	9	20 25	44	0.42	0.60	0.61	0.22	-	-	-	-	15.5
9	7	9	9	9	<2	-	0.32	0.74	0.75	0.22	0.22	0.22	0.22	0.22	16.4
9	7	9	9	9	2<3	59	0.26	0.44	0.41	0.22	-	-	-	-	16.4
9	7	9	9	9	3-5	54	0.22	0.35	0.35	0.22	-	-	-	-	16.4
9	7	9	9	9	10	49	0.24	0.37	0.39	0.22	-	-	-	-	16.4
9	7	9	9	9	15	44	0.31	0.48	0.51	0.22	-	-	-	-	16.4
9	7	9	9	9	20	44	0.39	0.62	0.65	0.22	-	-	-	-	16.4
9	8	9	9	9	<2	-	0.22	0.43	0.39	0.22	0.22	0.22	0.22	0.22	17.3
9	8	9	9	9	2<3	59	0.24	0.46	0.43	0.22	-	-	-	-	17.3
9	8	9	9	9	3-5	59	0.22	0.37	0.38	0.22	-	-	-	-	17.3
9	8	9	9	9	10	54	0.22	0.39	0.41	0.22	-	-	-	-	17.3
9	8	9	9	9	15	44	0.29	0.50	0.53	0.22	-	-	-	-	17.3
9	8	9	9	9	20	44	0.36	0.64	0.67	0.22	-	-	-	-	17.3
9	9	9	9	9	<2	-	0.22	0.44	0.42	0.22	0.22	0.22	0.22	0.22	18.2
9	9	9	9	9	2<3	72	0.23	0.49	0.46	0.22	-	-	-	-	18.2
9	9	9	9	9	3-5	72	0.22	0.39	0.40	0.22	-	-	-	-	18.2
9	9	9	9	9	10	59	0.22	0.40	0.43	0.22	-	-	-	-	18.2
9	9	9	9	9	15 20	49 49	0.27	0.51	0.55	0.22	-	-	-	-	18.2
9		-	9		20	43	0.34	0.00	0.03	0.22					10.2
		1	1		I	l	1		1	1	I	1	1	1	I

(1) For Box Length = 8'-0"

2) Asi thru Asa, Asa and Asa are minimum required areas of reinforcement per linear foot of box length. AS6 and AS5 are minimum required areas of reinforcement per linear foot of box width.

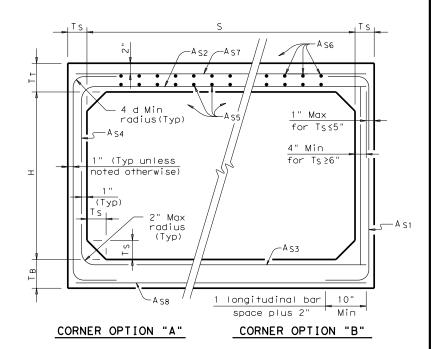


FILL HEIGHT 2 FT AND GREATER



SECTION A-A

(TOP AND BOTTOM SLAB JOINT REINFORCEMENT)



FILL HEIGHT LESS THAN 2 FT

GENERAL NOTES:

Designs shown conform to ASTM C1577. Refer to ASTM C1577 for information or details not shown.

All concrete shall be Class "H" Concrete with a minimum compressive strength of 5,000 psi. See SCP-MD standard sheet for miscellaneous

see SCP-MD standard sheet for miscellaneous details and notes not shown.

In lieu of furnishing the designs shown on this sheet, the contractor may furnish an alternate design that is equal to or exceeds the box design for the design fill height in the table. Shop plans for alternate designs shall be submitted in accordance with Item "Precast Concrete Structural Members (Fabrication)".

HL93 LOADING



SINGLE BOX CULVERTS PRECAST 9'-0" SPAN

SCP-9

FILE:	scp09sts.dgn	DN: GAF		ck: LMW	DW: B	NH/Txl	DOT	CK:	GAF
©TxD0T	February 2010	CONT	SECT	JOB	J0B		HIG	HWA	,
	REVISIONS								
		DIST		COUNT	Υ			SHEE	T NO.

TABLE OF DIMENSIONS & REINFORCING STEEL (Wings for One Structure End) Variable Dimensions Quantities\ Reinforcing per ft of wing length Bars Ji | Bars J2 Maximum (2~Wings) Winawal Reinf Conc Height Spa Spa (Lb/F+) (CY/F+ 2'-6" 2'- 5" 1'- 0" #4 1'-0" #4 1'-0" 33. 73 0. 248 3'-0" 2'- 5" 1'- 0" 9" 7" #4 1'-0" #4 1'-0" 37.07 0.261 #4 1'-0" #4 1'-0" 37.74 0.273 3'-6" 4'-0" 2'- 5" 1'- 0" 7" #4 1'-0" #4 1'-0" 38.41 0.285 7" #4 1'-0" #4 1'-0" 41.75 0.330 4'-6" 3'- 2" 1'- 6" 1'- 0" 5'-0" 3'- 2" 1'- 6" 1'- 0' 7" #4 1'-0" #4 1'-0" 45.09 0.343 #4 1'-0" #4 1'-0" 45.75 0.355 6'-0" 3'- 2" 1'- 6" 1'- 0" #4 1'-0" #4 1'-0" 46.42 0.367 7'-0" 3'- 8" 1'- 9" 1'- 3" 7" #4 1'-0" #4 1'-0" 52.77 0.414 #5 1'-0" #4 1'-0" 8'-0" 4'- 2" 2'- 0" 1'- 6" 60.19 | 0.486 9'-0" 4'- 8" 2'- 3" 1'- 9" 8" #4 6" #4 6" 81.49 0.535 10'-0" 5'- 2" 2'- 6" 2'- 0" 8" #5 6" #4 6" 97.25 0.584 11'-0" 5'- 8" 2'- 9" 2'- 3" 8" #6 6" #5 6" 133.65 0.634 12'-0" 6'- 2" 3'- 0" 2'- 6' 6" #5 162.29 0.721 13'-0" 6'- 8" 3'- 3" 2'- 9" 11" #7 6" | #5 | 6" 178.80 0.856 14'-0" 7'- 2" 3'- 6" 3'- 0" 1'- 0" #8 6" #5 6" 216.78 0.959 6" #6 15'-0" 7'- 8" 4'- 0" 3'- 0" 1'- 1" #9 6" 283.06 1.068 16'-0" 8'- 2" 4'- 6" 3'- 0" 1'- 3" #9 6" #6 6" 297.02 1.234 Finished Grade 9 (Roadway Slope) υ, Conforms to Slope perpendicular to Rdwy(4)I

TABLE OF WINGWALL REINFORCING

	(2~Wings)									
Bar	Size	No.	Spa							
DL	#5	~	1′-0"							
DS	#5	~	1′-0"							
Ε	#4	~	1′-0"							
F	#4	~	1′-0"							
G	#6	4	~							
М	#4	4	~							
Р	#4	~	1′-0"							
RS	#5	3	~							
RL	#5	3	~							
٧	#4	~	1′-0"							

TABLE OF ESTIMATED CULVERT TOEWALL

	QUANTITIES									
Bar	Size	No.	Spa							
L	#4	~	1′-6"							
Q	#4	1	~							
Rein	nf (Lb/	/F+)	2.45							
Cond	(CY/F	- +)	0.037							

WING DIMENSION CALCULATIONS:

Formulas: (All values are in Feet) Hw = H + T + C - 0.250A = (Hw - 0.333') (SL) $B = (A) [Tangen+ (\theta + 15^{\circ})]$ Lw = (A) \div [Cosine (θ + 15°)] For Cast-in-place culverts: $L+w = [(N) (S) + (N+1) (U)] \div (Cosine \Theta)$ For Precast culverts: L+w = $[(N) (2U + S) + (N - 1) (0.500')] \div (Cosine <math>\theta$) Total Wingwall Area (Two Wings \sim S.F.) = (0.5) (Hw + 0.333') (Lw + A)

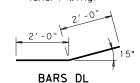
= Height of Wingwall

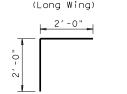
= Side Slope Ratio (Horizontal:1 Vertical)

Length of Short WingwallLength of Long Wingwall = Culvert Toewall Length = Number of Culvert Spans = Culvert Skew

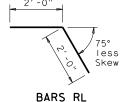
See applicable box culvert standard for H, S, T,







BARS RS (Short Wing)



(Long Wing)

BARS J2



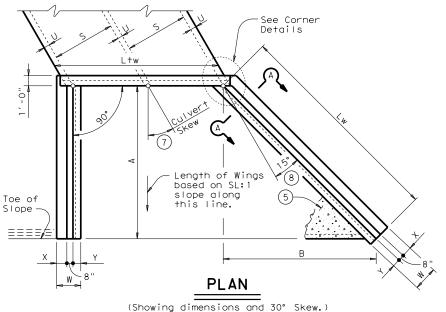
by $0.5 \times (A+Lw)$.

Box Culvert.

Adjust to fit as necessary to maintain 1 $\frac{1}{4}$ " clear cover and 4" minimum between bars. Quantities shown are based on an average wing height for two wings (one structure end). To determine total quantities for two wings multiply the tabulated values

Extend Bars P 3′-0" minimum into bottom slab of

- Recommended values of Slope are: 2:1, 3:1, 4:1, & 6:1.
 - When shown elsewhere on the plans, a 5" deep concrete riprap shall be constructed. Payment for riprap shall be as required by Item 432, "Riprap". Unless otherwise shown on the plans or directed by the Engineer, the riprap shall have a 6" wide by 1'-6" deep reinforced concrete toewall along all edges adjacent to natural ground; the toewall shall be reinforced by extending typical riprap reinforcing into the toewall; construction joints or grooved joints, oriented in the direction of flow, shall extend across the full distance of the riprap, at intervals of approximately 20'. When such riprap is provided, the culvert toewall shown in SECTION B-B will not be required.
- At Contractor's option, Culvert Toewall may be ended flush with Wingwall Toewall. Adjust reinforcing from that shown as necessary.
- Applicable values of of Skew are: 15°, 30°, and 45°.
- (8) Typical wingwall angle for all skews.
- 0" min to 5'-0" max. Estimated curb heights are shown elsewhere in the plans. For structures with pedestrian rail, bicycle rail or curbs taller than 1'-0", refer to ECD standard. For structures with T6 bridge rail, refer to T6-CM standard. For structures with traffic rail, other than T6, refer to RAC standard.
- For vehicle safety, curb heights and wall heights shall be reduced, if necessary, to provide a maximum 3" projection above finished grade. No changes will be made in quantities and no additional compensation will be allowed for this work.



Y + 4" **₹** BARS J1 BARS V W - 4"2'-0"

BARS L

GENERAL NOTES:

Designed according to AASHTO LRFD Specifications.

All reinforcing steel shall be Grade 60.
Synthetic fibers listed on the "Fibers for Concrete"
Material Producer List (MPL) may be used in lieu of steel reinforcing in riprap concrete unless noted otherwise.

All concrete shall be Class "C" and shall have a minimum compressive strength of 3600 psi.

All reinforcing bars shall be adjusted to provide a minimum of 1 $\frac{1}{4}$ " clear cover. When structure is founded on solid rock, depth of

toewalls for culverts and wingwalls may be reduced or eliminated as directed by the Engineer.

See BCS sheet for additional dimensions and information.

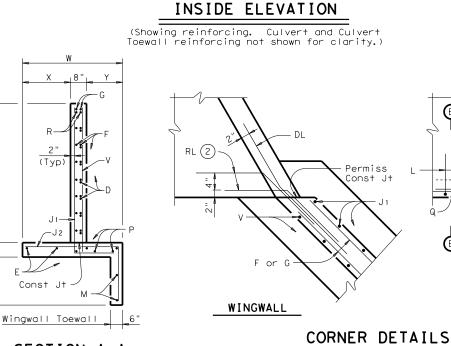
The quantities for concrete and reinforcing steel resulting from the formulas given on this sheet are for Contractor's information only.

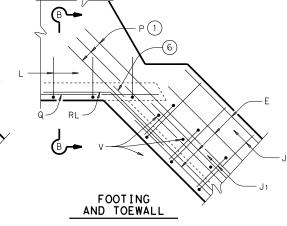


CONCRETE WINGWALLS WITH FLARED WINGS FOR SKEWED BOX CULVERTS

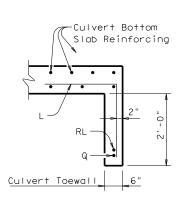
FW-S

fw-sstde.dgn	DN: GAF		CK: CAT	DW:	TxD0T	CK: GAF
xDOT February 2010	CONT	SECT	JOB	Н		SHWAY
REVISIONS						
-10: Add note for thetic fibers.	DIST		COUNTY			SHEET NO.





(Culvert and Culvert Toewall reinforcing not shown for clarity.)



SECTION B-B (5)

SECTION A-A

Finished

Grade —

Standard Line Post Installation

Toenail with one 16d Galv. nail to prevent block rotation. 31" 31" Finished 25" 25" Grade -Culvert Headwall-Post length Post Lenath 6'-0" 6'-0" (2) 3 ½" Dia. Holes 16" 40" 40"

Rectangular CRT Post

(6"x 8" x 6' Long)

(6) CRT required.

See Elevation Detail for locations.

GENERAL NOTES

- The type of line post (round wood post, rectangular wood post, or steel post) will
 be as shown in the plans. The exact position of the transitions shall be as shown
 in the plans or as directed by the Engineer. Steel posts to be galvanized in
 accordance with Item 445, "Galvanizing."
- 2. Rail element shall meet all requirements of Item 540, "Metal Beam Guard Fence" except as modified on the plans. The Contractor may furnish rail elements of 12 $\frac{1}{2}$ or 25 foot nominal lengths.
- 3. Rail post holes are offset 3'- 1 $\frac{1}{2}$ " from standard guardrail to accommodate the midspan splicing.
- 4. Button head post bolts (ASTM A307) shall be of sufficient length to extend through the full thickness of the nut (ASTM A563) and no more than $\frac{\gamma}{4}$ " beyond it. Button head splice bolts (ASTM A307) are $\frac{\gamma}{6}$ " x 1 $\frac{1}{4}$ " with a $\frac{\gamma}{6}$ " double recessed nut (ASTM A563). Galvanized fittings (bolts, nuts, and washers) shall be in accordance with Item, "Metal For Structures". Fittings shall be subsidiary to the bid item requiring construction of transition.
- 5. Where solid rock is encountered or where shown on the plans, the diameter of the holes shall be approximantley 12 inches, the backfilling shall be with a cohesionless material, and embedment depth shall be 1' 6" or more as directed by the Engineer.

DN: T×DOT CK: AM DW: VP

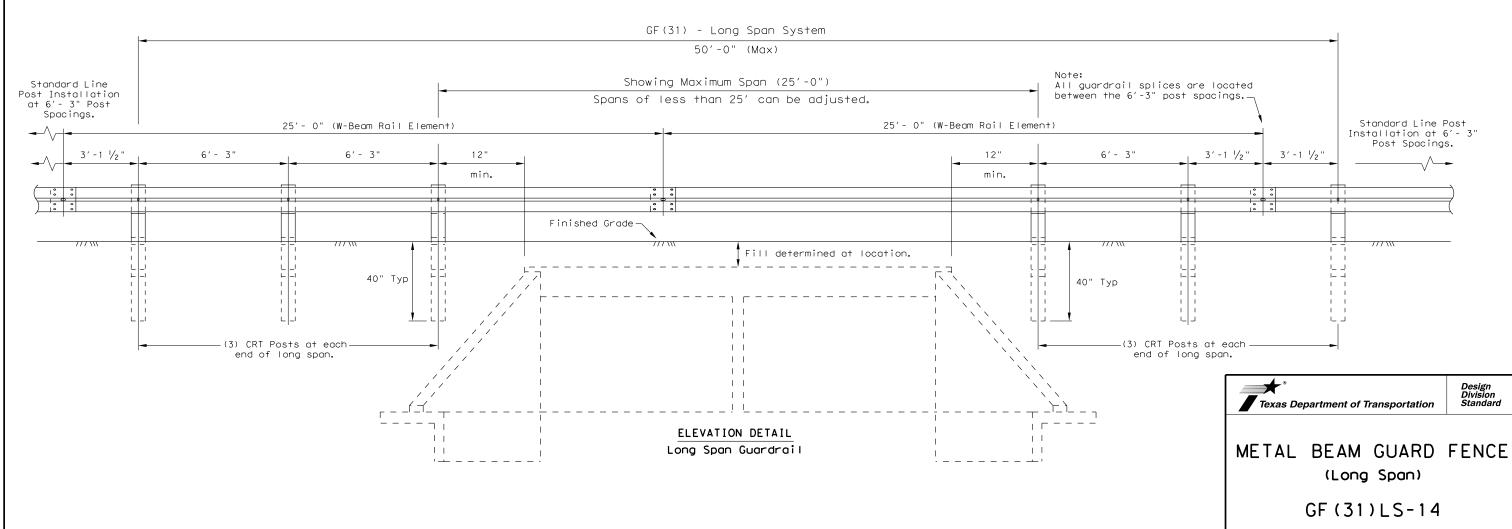
JOB

CONT SECT

FILE: gf31|s14.dgn C) TxDOT: December 2011 ck: CGL

HIGHWAY

- 6. Posts shall not be set in concrete, of any depth.
- 7. Refer to GF(31) Standard Sheet for additional details.
 - NOTE: Field drilled holes shall be repaired in accordance with Item 445, "Galvanizing". Flame cutting of holes in guardrail shall not be permitted.



Lateral Offset Between the

Guardrail and the Culvert Headwall

GENERAL NOTES

- 1. For additional information contact: Interstate Steel Inc. (432) 263-3725
- 2. The Type of SGT unit will be specified elsewhere in the plans. The numbers in the circles indicate post position. The Type of SGT unit chosen is a maintenance consideration and does not affect the systems performance. Post & Tube Options Post Only Post & Tube Options Post Only

Posts 3 thru 8 Posts 5 thru 8 Type I Posts (1) thru (2)
Type II Posts (1) thru (4)
Type III Posts (1) thru (8) None

- SGT's placed within the "minimum" 150 ft. radius, shall be installed straight. Standard rail elements may be installed within the radius, without special fabrication.
- 4. All bolts, nuts cable assemblies, cable anchors, steel tubes & bearing plates shall be galvanized.
- 5. A flare rate of 25:1 may be used over the first 50 ft. of the system to prevent the terminal head from encroaching the shoulder. The flare may be decreased or eliminated for specific installations, if directed by the Engineer.
- 6. The steel tubes shall not protrude more than 4 inches above ground. Site grading may be necessary to meet this requirement.
- 7. The steel tubes may be driven with an approved driving head. They shall not be driven with the wood post in the tube. If the steel tubes are placed in drilled holes, the backfill material must be satisfactorily compacted to prevent tube settlement.
- 8. If solid rock is encountered. See the Manufacturer's installation manual for the proper installation guidance.
- The breakaway cable assembly must be taut. A locking device, (vice grips or channel lock pliers) should be used to prevent the cable from twisting when tightening the nuts.
- 10. The wood blocks shall be "toe nailed" to the rectangular wood posts to prevent them from turning when the wood shrinks. The bearing plate on the front post shall also be "toe nailed" to prevent
- 11. For curb installations, the soil tubes and posts shall be installed at the proper ground elevation behind the curb. The posts will then require field drilling new holes to accommodate the rail to post connection bolt to maintain the proper height of the rail above the gutter pan. The excess post length above the rail will be removed if directed by the Engineer.
- 12. An object marker shall be installed on the front of the impact head as detailed on D&OM(VIA).

WOOD BLOCK

P675

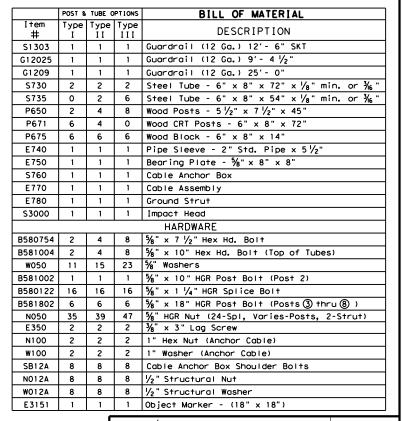
taken from bottom of posts.

P650

POST & TUBE OPTIONS

ſype III post① thru⑧

post (1) thru (2)





SINGLE GUARDRAIL TERMINAL (SKT-31)(WOOD POST)

SGT (8) 31-14

E: sgt83114.dgn	DN: Tx[)OT	CK: AM	CK: AM DW:		ск: VP
TxDOT December 2011	CONT	SECT	JOB		н	GHWAY
REVISIONS						
	DIST		COUNTY			SHEET NO.